

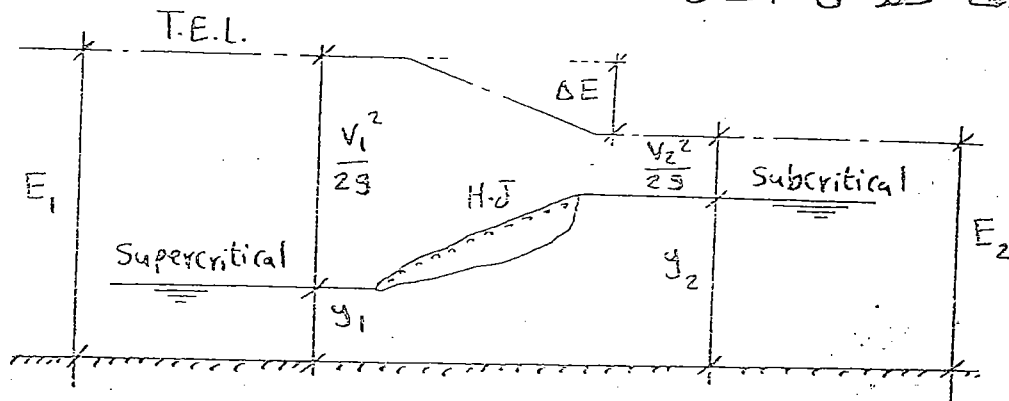
هيدروليكا  
ثانية مدني

***Rapidly Varied  
Flow  
(Hydraulic Jump)***

2014-2015

# Hydraulic Jump

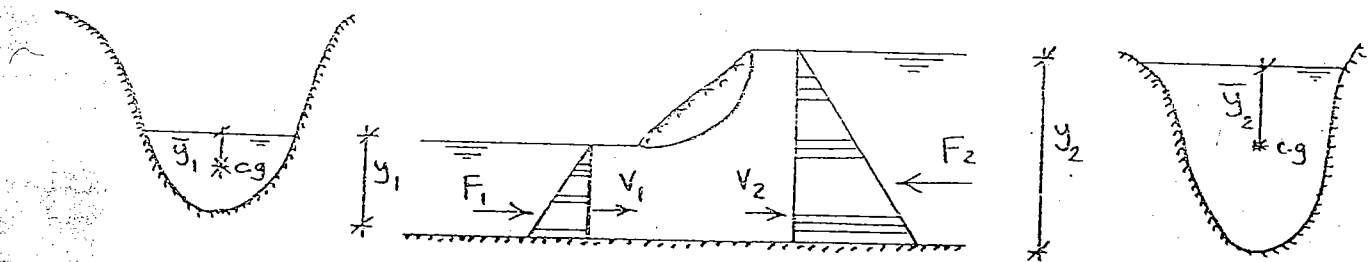
هي ظاهرة يتحول عندها ال flow من Subcritical إلى Supercritical ويحدث عندها فقد في الطاقة



To get  $y_1, y_2$

Apply the Momentum equation

$$\Sigma \text{ Forces} = \text{Rate of change in Momentum}$$



$$F_1 - F_2 = \rho Q V_2 - \rho Q V_1$$

$$\gamma A_1 \bar{y}_1 - \gamma A_2 \bar{y}_2 = \frac{\gamma}{g} Q \frac{Q}{A_2} - \frac{\gamma}{g} Q \frac{Q}{A_1}$$

$$A_1 \bar{y}_1 - A_2 \bar{y}_2 = \frac{Q^2}{g A_2} - \frac{Q^2}{g A_1}$$

$$A_1 \bar{y}_1 + \frac{Q^2}{g A_1} = A_2 \bar{y}_2 + \frac{Q^2}{g A_2} = \text{Const} = \text{Specific Force}$$

General case

## The Specific Force

(2)

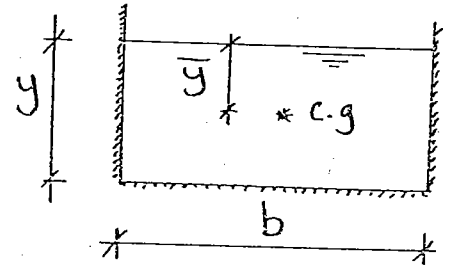
It is the Sum of both the Hydrostatic Force of water per unit Weight and the Momentum of flow passing through the channel per unit time per Unit Weight

$$F = \underbrace{A \bar{y}}_{\text{Hydrostatic}} + \underbrace{\frac{Q^2}{gA}}_{\text{Momentum}}$$

For rectangular section

$$A = by, \quad \bar{y} = \frac{y}{2}$$

$$Q = qb, \quad F = fb$$



Where  $f$  = specific force / unit length

$$fb = by \cdot \frac{y}{2} + \frac{q^2 b^2}{gby}$$

$$f = \frac{y^2}{2} + \frac{q^2}{gy}$$

Rectangular section only

Dimensions

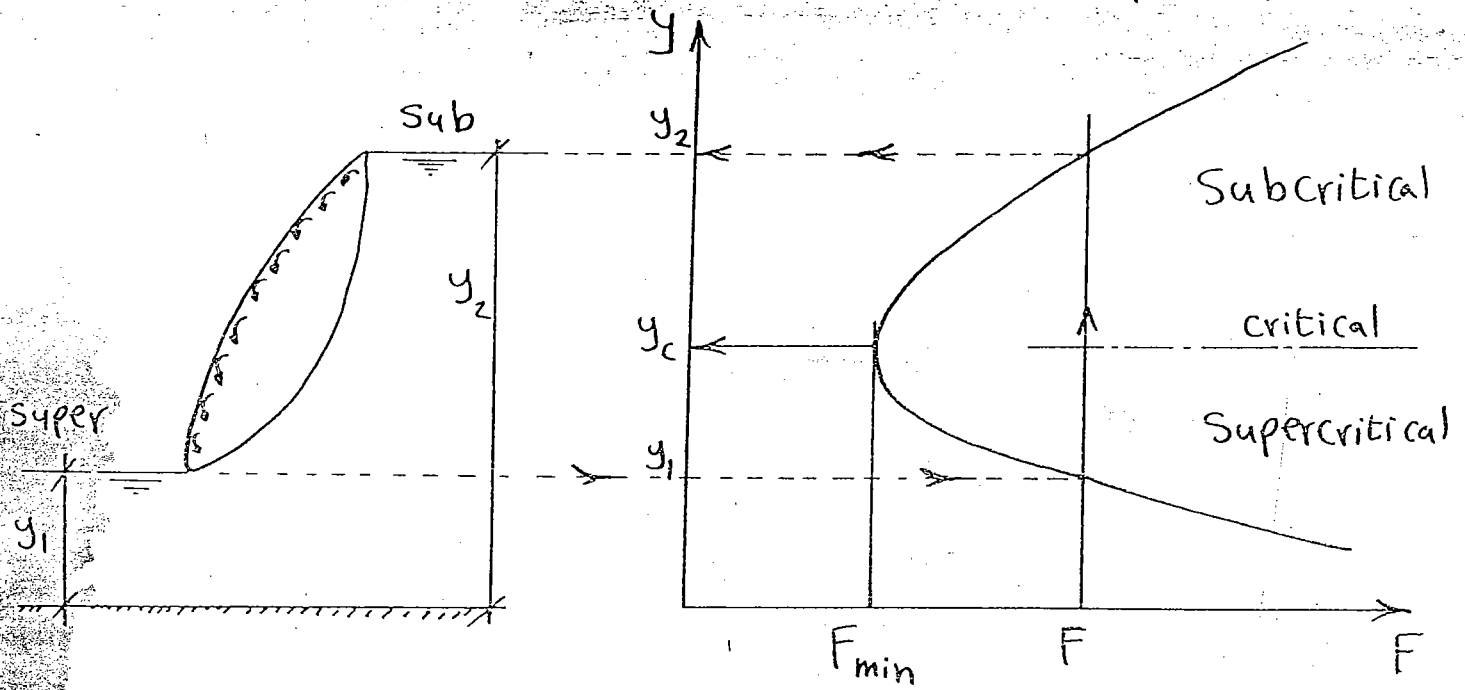
$$F : L^3$$

$$f : L^2$$

$$\text{Force} = F \gamma$$

$$\text{Force} = f b \gamma$$

# The Specific Force Diagram (F-y) ③



$y_1, y_2$  are two Sequent depths (Conjugate depths)

They have the same Specific Force  $F$  & same Discharge  $Q$ .

One is Subcritical  $y_2$  and the other is Supercritical  $y_1$

$y_1$ : initial depth

$y_2$ : sequent depth

Minimum Specific Force ( $F_{min}$ ) for a rectangular section

$$F = \frac{y^2}{2} + \frac{q^2}{gy}$$

for  $F_{min}$   $\frac{dF}{dy} = 0$

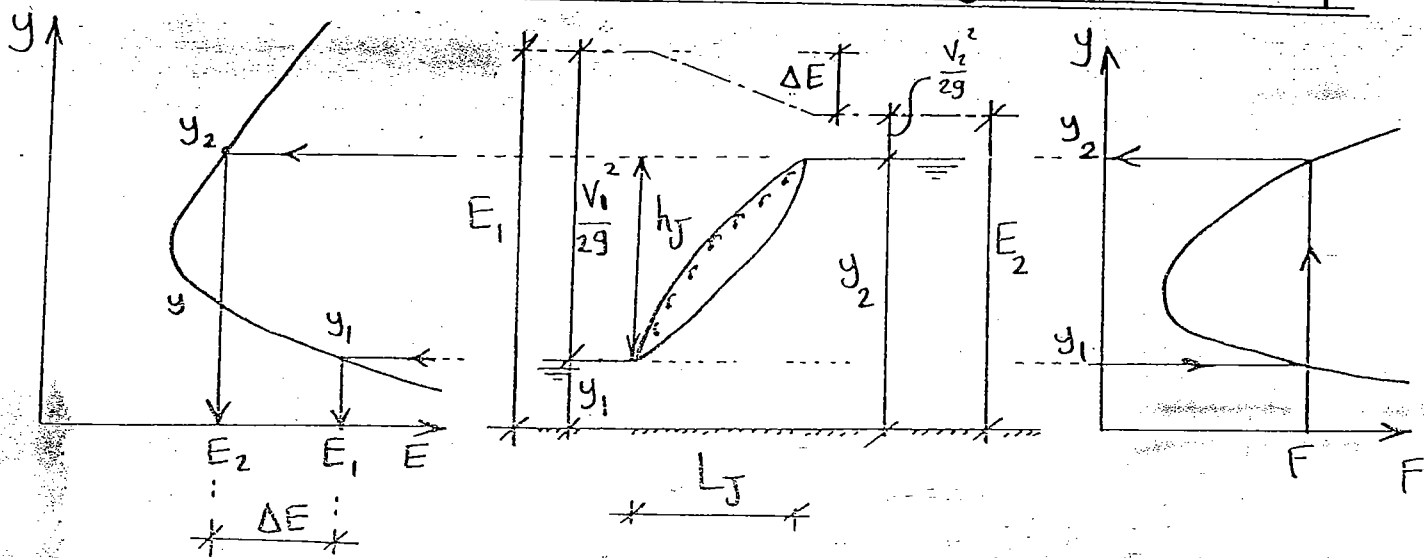
$$\frac{dF}{dy} = \frac{2y}{2} - \frac{q^2}{gy^2} = 0$$

$$y = \frac{q^2}{g}$$

$\Rightarrow$

$$y_c = \sqrt[3]{\frac{q^2}{g}}$$

# Characteristics of the Hydraulic Jump (4)



- ①  $y_1$  = initial depth
- ②  $y_2$  = sequent depth
- ③  $h_J$  = height of Jump  $h_J = y_2 - y_1$
- ④  $L_J$  = Length of Jump  $L_J = 6y_2$
- ⑤  $\Delta E$  = Energy loss  $\Delta E = E_1 - E_2$
- ⑥  $\eta$  = Efficiency  $\eta = \frac{E_2}{E_1} = \frac{\text{Output}}{\text{input}}$
- ⑦  $HP$  = horse power lost  $HP = \frac{\gamma Q \Delta E}{K}$

## Relative Characteristics of H.J

$$\frac{y_1}{E_1}, \frac{y_2}{E_1}, \frac{h_J}{E_1}, \frac{L_J}{E_1}, \frac{\Delta E}{E_1}, \frac{E_2}{E_1}, HP$$

$\gamma$	1000	9810	62.4
K	75	735	550

# Hydraulic Jump in a Rectangular Section

General:

$$A_1 \bar{y}_1 + \frac{Q^2}{g A_1} = A_2 \bar{y}_2 + \frac{Q^2}{g A_2}$$

$$\cancel{b} y_1 \frac{y_1}{2} + \frac{q^2 \cancel{b}^2}{g \cancel{b} y_1} = \cancel{b} y_2 \frac{y_2}{2} + \frac{q^2 \cancel{b}^2}{g \cancel{b} y_2}$$

$$\frac{1}{2} y_1^2 + \frac{q^2}{g y_1} = \frac{1}{2} y_2^2 + \frac{q^2}{g y_2}$$

$$\frac{1}{2} (y_1^2 - y_2^2) = \frac{q^2}{g} \left( \frac{1}{y_2} - \frac{1}{y_1} \right)$$

$$\frac{1}{2} (\cancel{y_1 - y_2}) (y_1 + y_2) = \frac{q^2}{g} \left( \frac{\cancel{y_1 - y_2}}{y_1 y_2} \right)$$

$$\boxed{\frac{1}{2} (y_1 + y_2) = \frac{q^2}{g y_1 y_2}} \rightarrow \textcircled{1}$$

As a function of critical depth

$$\therefore y_c^3 = \frac{q^2}{g}$$

$$\therefore \frac{1}{2} (y_1 + y_2) = \frac{y_c^3}{y_1 y_2}$$

$$\boxed{\frac{1}{2} y_1 y_2 (y_1 + y_2) = y_c^3}$$

rect - sec only

As a function of Froude number 1

(6)

$$\boxed{y_1 + y_2 = \frac{2q^2}{gy_1 y_2}} \rightarrow \textcircled{1} \quad \therefore q = V_1 y_1$$

$$y_1 + y_2 = \frac{2V_1^2 y_1^2}{gy_1 y_2}$$

$$y_1 + y_2 = \frac{2V_1^2}{g} \frac{y_1}{y_2} \quad * \frac{y_2}{y_1^2}$$

$$\left(\frac{y_2}{y_1}\right)^2 + \left(\frac{y_2}{y_1}\right) = \frac{2V_1^2}{gy_1^2} = 2F_1^2$$

$$\left(\frac{y_2}{y_1}\right)^2 + \left(\frac{y_2}{y_1}\right) - 2F_1^2$$

$$\frac{y_2}{y_1} = \frac{-1 \pm \sqrt{1 + 8F_1^2}}{2}$$

$$\frac{y_2}{y_1} = \frac{1}{2} \left( \sqrt{1 + \frac{8q^2}{gy_1^3}} - 1 \right)$$

$$\boxed{\frac{y_2}{y_1} = \frac{1}{2} \left( \sqrt{1 + 8F_1^2} - 1 \right)}$$

rect - sec (only)

As a function of Froude number 2

$$\boxed{y_1 + y_2 = \frac{2q^2}{gy_1 y_2}} \rightarrow \textcircled{1} \quad \therefore q = V_2 y_2$$

$$y_1 + y_2 = \frac{2V_2^2 y_2^2}{gy_1 y_2} \quad * \frac{y_1}{y_2^2}$$

$$\left(\frac{y_1}{y_2}\right) + \left(\frac{y_1}{y_2}\right) = \frac{2V_2^2}{gy_2} = 2F_2^2$$

$$\boxed{\frac{y_1}{y_2} = \frac{1}{2} \left( \sqrt{1 + 8F_2^2} - 1 \right)}$$

rect - sec (only)

# Types of Jumps

(7)

According to normal water depth  $y_n$

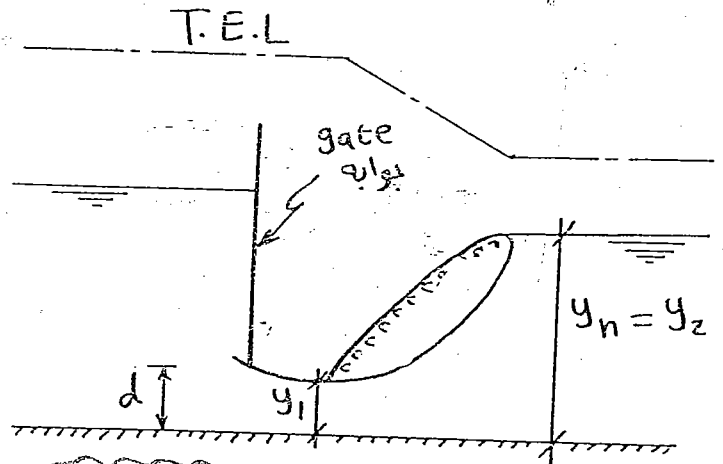
From Manning  
or given  
or measured  
in lab

$$y_2 = \frac{y_1}{2} \left( \sqrt{1 + 8F_1^2} - 1 \right)$$

$$\Rightarrow y_2 = y_1$$

If  $y_n = y_2$

Perfect Jump



$$y_1 = C_c d$$

at Vena Contracta  
 $d \equiv$  gate opening

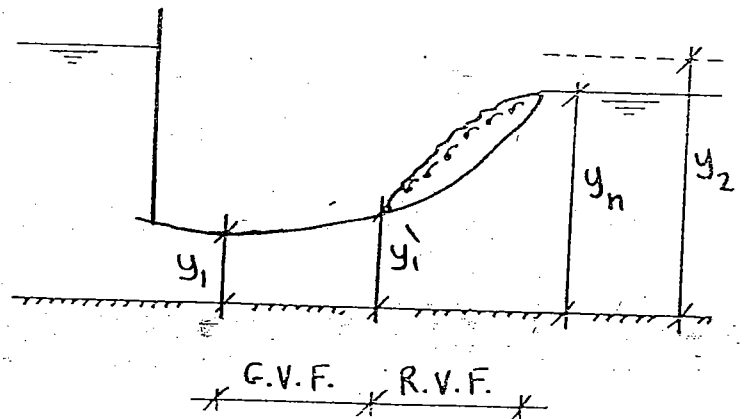
If  $y_n < y_2$

Repelled Jump

Hyd Jump  $y_1$   $y_2$   $y_n$

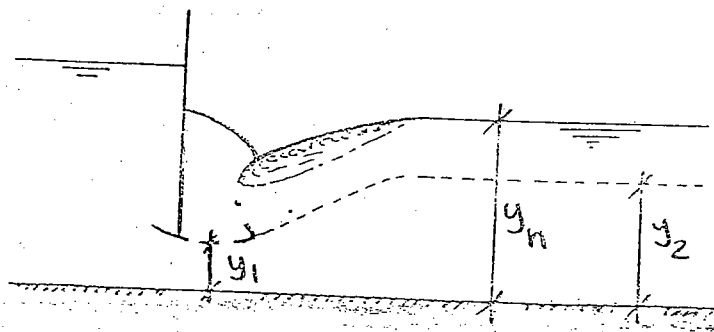
$$\frac{y_1}{y_n} = \frac{1}{2} \left( \sqrt{1 + 8F_n^2} - 1 \right)$$

$$\Rightarrow y_1 = y_n$$



If  $y_n > y_2$

Submerged Jump





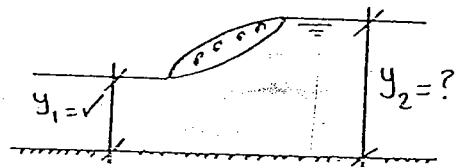
Given: Rectangular Section

(8)

If  $y_1$  is given and  $y_2$  is unknown

$$\frac{y_2}{y_1} = \frac{1}{2} \left( \sqrt{1 + 8F_1^2} - 1 \right)$$

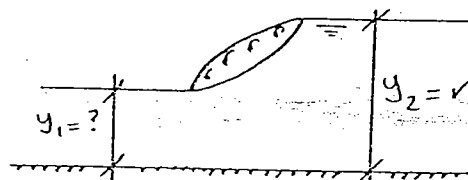
$$F_1 = \frac{V_1}{\sqrt{g y_1}}$$



If  $y_2$  is given and  $y_1$  is unknown

$$\frac{y_1}{y_2} = \frac{1}{2} \left( \sqrt{1 + 8F_2^2} - 1 \right)$$

$$F_2 = \frac{V_2}{\sqrt{g y_2}}$$



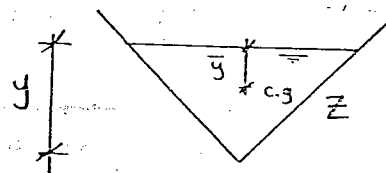
For any section

$$F = \frac{Q^2}{g A_1} + A_1 \bar{y}_1 = \frac{Q^2}{g A_2} + A_2 \bar{y}_2$$

Triangular Section

$$A = Z y^2$$

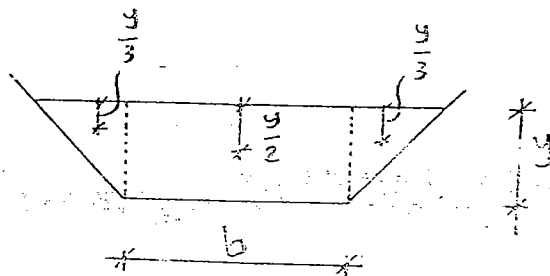
$$\bar{y} = \frac{y}{3}$$



Trapezoidal Section

$$A = b y + Z y^2$$

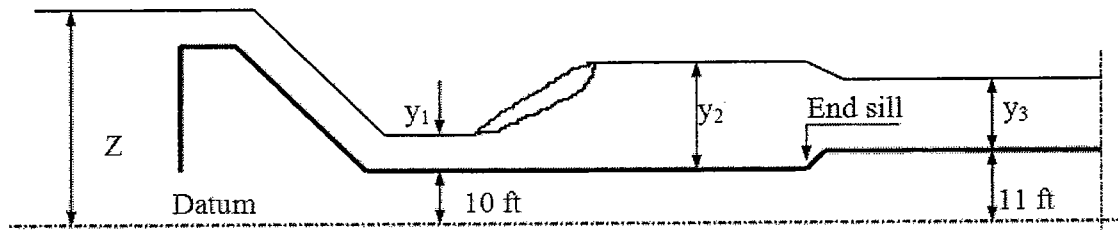
$$A \bar{y} = b y \left( \frac{y}{2} \right) + Z y^2 \left( \frac{y}{3} \right)$$





**ASSIGNMENT NO. 7**  
***Rapidly Varied Flow***

1. Define alternate and sequent depths as used in connection with open channel flow. Plot the specific force diagram and specific energy diagram for a 10 m rectangular channel carrying 15 m<sup>3</sup>/sec. Determine the critical depth, the minimum specific energy and minimum specific force. Use both diagrams to determine the initial depth corresponding to a sequent depth 1.5 m, the energy loss in the jump and the height of the jump.
2. A spillway discharge a flood flow at a rate of 7.75 m<sup>3</sup>/s/m. At the downstream horizontal apron the depth of flow was found to be 0.50 m. What tailwater depth is needed to form a hydraulic jump? If a jump is formed, find its
  - a) Type
  - b) Length
  - c) Headloss
  - d) Energy loss as a percentage of the initial energy
3. A hydraulic jump occurs in a rectangular open channel. The water depths before and after the jump are 0.6 m and 1.5 m, respectively. Calculate the critical depth and the energy loss.
4. A sluice gate is 3.0 m wide rectangular horizontal channel releases a discharge of 18.0 m<sup>3</sup>/s. The gate opening is 0.67 m and coefficient of contraction can be assumed to be 0.60. Examine the type of hydraulic jump formed when the tailwater depth is 3.60 m, 5.00m, and 4.09 m.
5. Water flows in wide, finished concrete channel with  $n = 0.014$ . Such that a hydraulic jump occurs at the transition of the change in slope of the channel bottom. If the upstream Froude number and depth are 4.0 and 6.0 cm, respectively, determine the slopes upstream and downstream of the jump to maintain uniform flows in those regions. Compare these slopes with the critical slope. The jump can be treated as a jump on horizontal surface.
6. A discharge of 50 m<sup>3</sup>/s flows in a trapezoidal channel having a bottom width of 4 m and side slope 1:1, if the sequent depth of the jump is 4 m. Determine the following:
  - a) The initial depth.
  - b) The initial Froude number.
  - c) The height of the jump.
  - d) The energy before and after the jump.
  - e) The horsepower lost in the jump.
7. a) Water discharges at the rate of 8000 ft<sup>3</sup>/sec over a spillway 40 ft wide into a stilling basin of the same width. The lake level behind the spillway is 100 ft above datum. The bed level downstream is 10 ft above datum. Assuming no energy dissipated in the flow down the spillway, find the initial depth  $y_1$  and the sequent depth  $y_2$  required for a hydraulic jump to form within the basin. Write down the main characteristics of the jump. Crest, stilling basin and downstream channel all have the same width.



b) For a different lake level behind the spillway, for a discharge of 8000 cfs and crest level of 40 ft,  $y_2$  is found to be 30 ft.

- i. Assume that the end sill is a smooth transition without losses and calculate  $y_3$ . Calculate the force on the sill. Specify units.
- ii. Suppose the sill is not a smooth transition and that laboratory tests indicate that the force on the sill is given by  $f = 100 V_2^2 h$  which  $f$  = force in pounds per foot width  
 $V_2$  = velocity in ft/s  
 $h$  = sill height in feet  
 Calculate  $y_3$  and find the head loss across the sill

Question(1):-

Assignment (7)

2014/2015

$$Q = 15 \text{ m}^3/\text{s} \quad B = 10 \text{ m}$$

$$\textcircled{1} E = y + \frac{Q^2}{2gB^2 y^2}$$

$$F = A\bar{y} + \frac{Q^2}{gA}$$

$$E = y + \frac{(15)^2}{2(9.81)(10)^2 y^2}$$

$$F = 10 y \frac{y}{2} + \frac{(15)^2}{9.81(10) y}$$

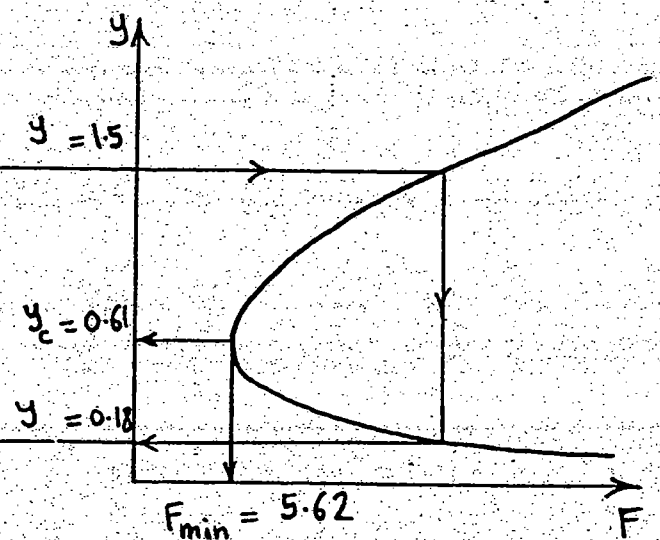
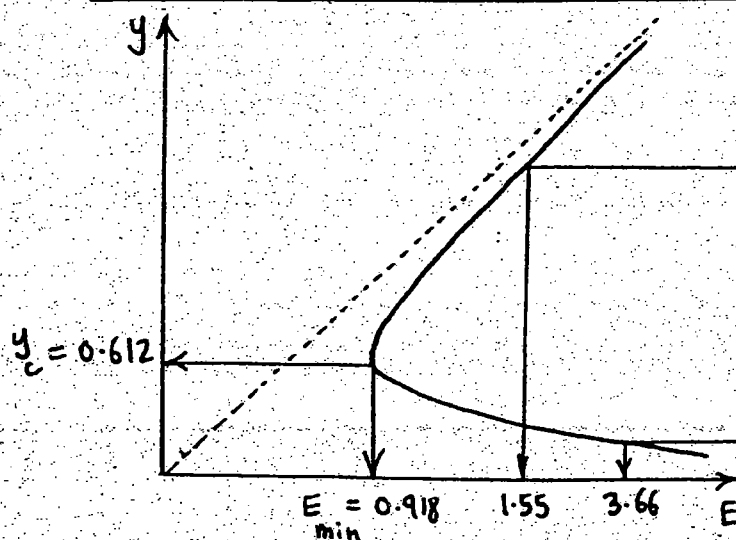
$$E = y + \frac{1}{8.72 y^2}$$

$$F = 5y^2 + \frac{2.293}{y}$$

$$q = \frac{Q}{b} = \frac{15}{10} = 1.5$$

$$y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{(1.5)^2}{9.81}} = \underline{\underline{0.612 \text{ m}}}$$

y	0.2	0.4	0.5	0.612	0.7	0.9	1.50	3.00
E	3.06	1.12	0.96	0.918	0.934	1.04	1.55	3.01
F	11.66	6.53	5.83	5.62	5.72	6.6	12.7	45.7

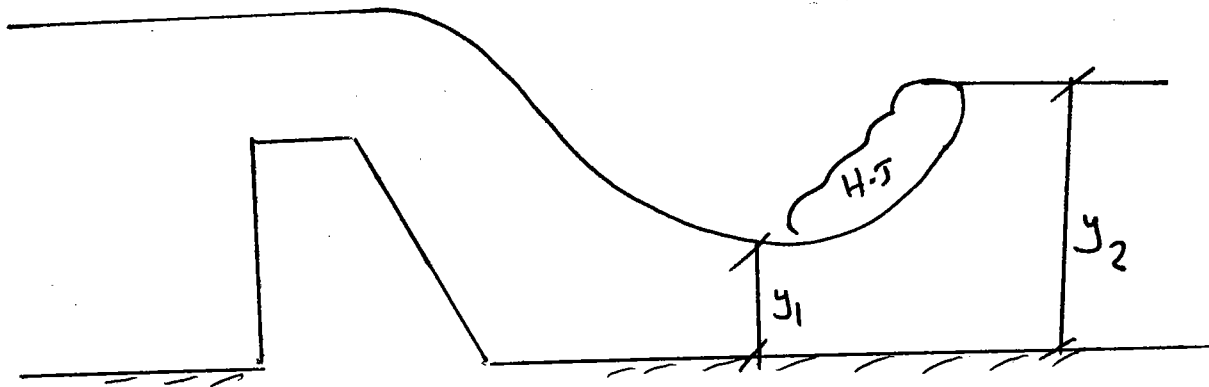


$$y = 1.5 \Rightarrow y = \underline{\underline{0.18 \text{ m}}}$$

$$\Delta E = E - E = 3.66 - 1.55 = \underline{\underline{2.11 \text{ m}}}$$

$$h_f = y - y = 1.5 - 0.18 = \underline{\underline{1.32 \text{ m}}}$$

②  $q = 7.75 \text{ m}^3/\text{s}/\text{m}$



$$\frac{y_2}{y_1} = \frac{1}{2} \left( \sqrt{1 + 8F_{N1}^2} - 1 \right)$$

$$F_{N1} = \frac{V_1}{\sqrt{gy_1}}, \quad V_1 = \frac{q}{y_1} = \frac{7.75}{0.5} = 15.5 \text{ m/s}$$

$$\therefore F_{N1} = \frac{15.5}{\sqrt{9.81 \times 0.5}} = 7 > 1 \quad (\text{Super Critical})$$

$$\Rightarrow \frac{y_2}{0.5} = \frac{1}{2} \left( \sqrt{1 + 8(7)^2} - 1 \right) \Rightarrow y_2 = 4.7 \text{ m}$$

$$V_2 = \frac{q}{y_2} = \frac{7.75}{4.7} = 1.65 \text{ m/s}$$

$$F_{N2} = \frac{1.65}{\sqrt{9.81 \times 4.7}} = 0.24 < 1 \quad (\text{Sub Critical})$$

②

∴ Flow Converts from Super Critical to Subcritical

⇒ Hydraulic jump will occur

a) Type of jump is perfect  $y_1 = y_2$  \*

b)  $L_j = 6y_2 = 6 \times 4.7 = 28.2^m$

c)  $h_L = \Delta E = E_1 - E_2$

$$E_1 = y_1 + \frac{V_1^2}{2g} = 0.5 + \frac{(15.5)^2}{2 \times 9.81} = 12.75 \text{ m}$$

$$E_2 = y_2 + \frac{V_2^2}{2g} \quad \& \quad V_2 = \frac{Q}{y_2} = \frac{7.75}{4.7} = 1.65 \text{ m}$$

$$E_2 = 4.7 + \frac{(1.65)^2}{2 \times 9.81} = 4.83 \text{ m}$$

$$\therefore h_L = \Delta E = 12.75 - 4.83 = \underline{\underline{7.92^m}}$$

d)  $\frac{h_L}{E_1} = \frac{7.92}{12.75} \times 100 = \underline{\underline{62.11 \%}}$

$$\textcircled{3} y_1 = 0.6^m$$

$$y_2 = 1.5^m$$

Req  $y_c$

$$\frac{y_1}{y_2} = \frac{1}{2} \left( \sqrt{1 + 8 F_{N_2}^2} - 1 \right)$$

$$\frac{0.6}{1.5} = \frac{1}{2} \left( \sqrt{1 + 8 F_{N_2}^2} - 1 \right)$$

$$\therefore F_{N_2} = 0.53$$

$$\therefore F_{N_2} = \frac{V_2}{\sqrt{g y_2}} \Rightarrow 0.53 = \frac{V_2}{\sqrt{9.81 \times 1.5}}$$

$$\therefore V_2 = 2 \text{ m/s}$$

$$q = y_2 V_2 = 1.5 \times 2 = 3 \text{ m}^3/\text{s/m}$$

$$\therefore y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{(3)^2}{9.81}}$$

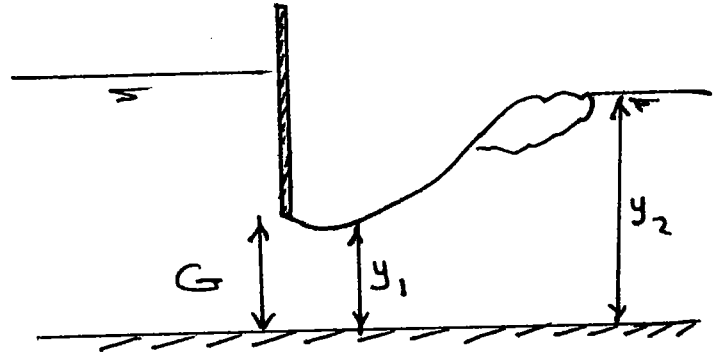
$$y_c = 0.97^m$$

$$\textcircled{4} b = 3^m$$

$$Q = 18 \text{ m}^3/\text{s}$$

$$G = 0.67^m$$

$$C_c = 0.6$$



$$y_1 = C_c \times G = 0.6 \times 0.67 = 0.4^m$$

$$q = \frac{Q}{b} = \frac{18}{3} = 6 \text{ m}^3/\text{s}/\text{m}$$

$$V_1 = \frac{q}{y_1} = \frac{6}{0.4} = 15 \text{ m/s}$$

$$F_{N1} = \frac{V_1}{\sqrt{gy_1}} = \frac{15}{\sqrt{9.81 \times 0.4}} = 7.57$$

$$y_2 = \frac{y_1}{2} \left( \sqrt{1 + 8(F_{N1})^2} - 1 \right) = \underline{\underline{4.09^m}}$$

let the tail race water depth =  $y_n$

i)  $y_n = 3.6^m \Rightarrow y_2 > y_n \Rightarrow$  repelled jump

ii)  $y_n = 5^m \Rightarrow y_2 < y_n \Rightarrow$  submerged jump

iii)  $y_n = 4.09^m \Rightarrow y_2 = y_n \Rightarrow$  Perfect jump

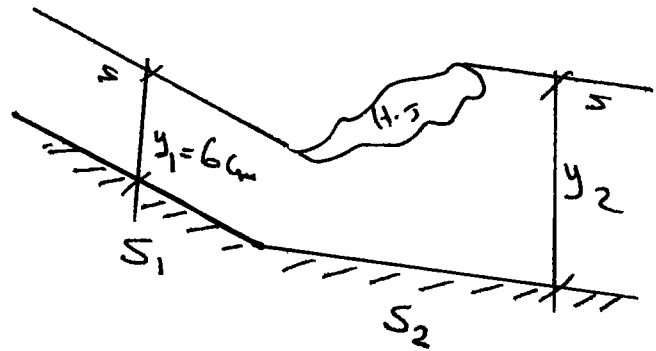


$$\eta = 0.014, F_{N_1} = 4$$

$$(5) S_1 = ?, S_2 = ?, S_c = ?$$

$$F_{N_1} = \frac{V_1}{\sqrt{g y_1}}$$

$$4 = \frac{V_1}{\sqrt{9.81 \times 0.06}} \Rightarrow V_1 = 3.07 \text{ m/s}$$



$$q = V_1 y_1 = 3.07 \times 0.06 = 0.184 \text{ m}^3/\text{s/m}$$

$$V_1 = \frac{1}{n} R_1^{2/3} S_1^{1/2}, \text{ for wide channel } R = y$$

$$\therefore 3.07 = \frac{1}{0.014} (0.06)^{2/3} S_1^{1/2} \Rightarrow S_1 = 0.0786$$

$$\frac{y_2}{y_1} = \frac{1}{2} (\sqrt{1 + 8 F_{N_1}^2} - 1)$$

$$y_2 = \frac{0.06}{2} (\sqrt{1 + 8(4)^2} - 1) = 0.31 \text{ m}$$

$$V_2 = \frac{q}{y_2} = \frac{0.184}{0.31} = 0.59 \text{ m/s}$$

$$V_2 = \frac{1}{n} R_2^{2/3} S_2^{1/2}$$

$$0.59 = \frac{1}{0.014} (0.31)^{2/3} (S_2)^{1/2} \Rightarrow S_2 = 3.3 \times 10^{-4}$$

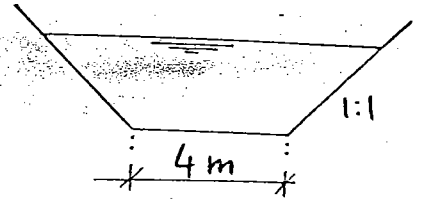
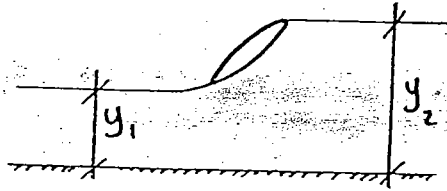
$$y_c = \sqrt[3]{\frac{q^2}{g}} = \sqrt[3]{\frac{(0.184)^2}{9.81}} = 0.15 \text{ m}$$

$$S_c = \frac{g n^2}{y_c^{1/3}} = \frac{9.81 (0.014)^2}{(0.15)^{1/3}} = 3.6 \times 10^{-3}$$

$S_1 > S_c \Rightarrow \text{steep slope}$  &  $S_2 < S_c \Rightarrow \text{Mild slope}$

⑥  $Q = 50 \text{ m}^3/\text{s}$

$y_2 = 4 \text{ m}$



$$A_1 \bar{y}_1 + \frac{Q^2}{g A_1} = A_2 \bar{y}_2 + \frac{Q^2}{g A_2}$$

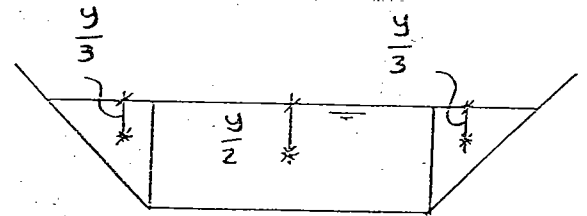
$A_1 = 4y + y^2$

$A_2 = 4(4) + (4)^2 = 32 \text{ m}^2$

$A \bar{y} = \int y^2 \left(\frac{y}{3}\right) + b y \left(\frac{y}{2}\right)$

$A_1 \bar{y}_1 = \frac{y_1^3}{3} + \frac{4}{2} y_1^2 = \frac{y_1^3}{3} + 2 y_1^2$

$A_2 \bar{y}_2 = \frac{(4)^3}{3} + 2(4)^2 = 53.33 \text{ m}^3$



$$\therefore \frac{y_1^3}{3} + 2 y_1^2 + \frac{(50)^2}{9.81(4y_1 + y_1^2)} = 53.33 + \frac{(50)^2}{9.81(32)}$$

$$\frac{y_1^3}{3} + 2 y_1^2 + \frac{254.84}{4y_1 + y_1^2} = 61.294$$

سolves by trial & error

$\Rightarrow y_1 = \underline{0.878 \text{ m}}$

$\Rightarrow A_1 = 4(0.878) + (0.878)^2 = 4.28 \text{ m}^2$

$V_1 = \frac{Q}{A_1} = \frac{50}{4.28} = 11.67$

$T_1 = B + 2zy_1 = 4 + 2(1)(0.878) = 5.756$

$D_1 = \frac{A_1}{T_1} = \frac{4.28}{5.756} = 0.743$

⑧

$$F_1 = \frac{V_1}{\sqrt{gD_1}} = \frac{11.67}{\sqrt{9.81(0.743)}} = \underline{\underline{4.32}} > 1 \quad \text{Supercritical}$$

$$h_T = y_2 - y_1 = 4 - 0.878 = \underline{\underline{3.122}}$$

$$E_1 = y_1 + \frac{Q^2}{2gA_1^2} = 0.878 + \frac{(50)^2}{2(9.81)(4.28)^2} = \underline{\underline{7.83}}$$

$$E_2 = y_2 + \frac{Q^2}{2gA_2^2} = 4 + \frac{(50)^2}{2(9.81)(32)^2} = \underline{\underline{4.12}}$$

$$\Delta E = E_1 - E_2 = 7.83 - 4.12 = 3.7 \text{ m}$$

$$HP = \frac{\gamma Q \Delta E}{K} = \frac{1000(50)(3.7)}{75} = \underline{\underline{2470 \text{ HP}}} \quad \textcircled{9}$$

The diagram illustrates a river channel with a dam and a spillway. The channel bed profile is shown with a total height of 100' from the datum to the top of the dam. The dam width is 40'. The spillway height is 10'. The downstream channel width is 11'. The water surface profile is labeled with  $y_1$ ,  $y_2$ , and  $y_3$  at different sections. The channel bed is labeled with  $y_0^3/2y$ . The diagram is labeled 'E1' and 'S11'.

$$\therefore E_0 = E_1 + \frac{q^2}{2gy_1^2}$$

$$\therefore (y_1 = 2.66 \text{ Ft}) \Rightarrow V_1 = \frac{q}{y_1} = \frac{200}{2.66} = 75.2 \text{ Ft/s}$$

$$y_2 = \frac{2.66}{2} \left( \sqrt{1 + 8(8.12)^2} - 1 \right) = 29.26 \text{ ft}$$

$$h_j = y_2 - y_1 = 29.26 - 2.66 = 26.58 \text{ ft}$$

$$h_L = 0 \quad \& \quad \Delta E = 0$$

①

b)  $Q = 8000 \text{ ft}^3/\text{s}$

$y_2 = 30 \text{ ft}$

i) Smooth end sill  $\rightarrow$  No Energy losses on it

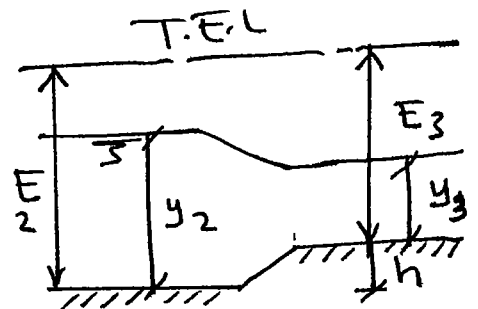
\* we can assume it as a hump

$E_2 = E_3 + h$

$y_2 + \frac{q^2}{2gy_2^2} = y_3 + \frac{q^2}{2gy_3^2} + 1$

$30 + \frac{(200)^2}{2 \times 32.2 \times (30)^2} = y_3 + \frac{(200)^2}{2 \times 32.2 (y_3^2)} + 1$

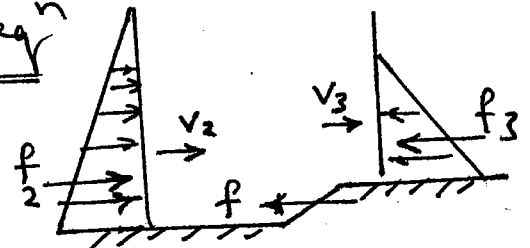
$29.7 = y_3 + \frac{621}{y_3^2} \Rightarrow y_3 = 5 \text{ ft}$



Force on the sill:-

apply momentum eqn

$F_2 - F_3 - F = \rho Q (V_3 - V_2)$



$\gamma A_2 \bar{y}_2 - \gamma A_3 \bar{y}_3 - F = 1.94 Q (V_3 - V_2)$

we can divide this eqn on  $\gamma$

$\therefore \underbrace{\gamma y_2 \left(\frac{y_2}{2}\right)}_{f_2} - \underbrace{\gamma y_3 \left(\frac{y_3}{2}\right)}_{f_3} - f = 1.94 q (V_3 - V_2)$

$62.4 (30) \left(\frac{30}{2}\right) - 62.4 (5) \left(\frac{5}{2}\right) - f = 1.94 \times 200 \left(\frac{200}{5} - \frac{200}{30}\right)$

$\Rightarrow f = 14366.67 \text{ lb/ft}$

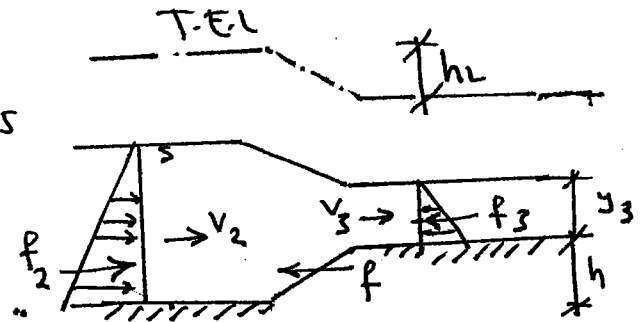
$$(i) f = 100 V_2^2 h$$

Req  $y_3$ ,  $h_L$  on the sill

$$V_2 = \frac{q}{y_2} = \frac{200}{30} = 6.67 \text{ ft/s}$$

$$h = 11 - 10 = 1 \text{ ft}$$

$$\therefore f = 100 (6.67)^2 \times 1 = 4448.9 \text{ lb/ft}$$



$$f_2 - f_3 - f = 34 (V_3 - V_2)$$

$$8 y_2 \left( \frac{y_2}{2} \right) - 8 y_3 \left( \frac{y_3}{2} \right) - 4448.9 = 34 \left( \frac{200}{y_3} - 6.67 \right)$$

$$62.4 \times 30 \times \frac{30}{2} - 62.4 \times \frac{y_3^2}{2} - 4448.9 = 1.94 \times 200 \left( \frac{200}{y_3} - 6.67 \right)$$

$$23631 - 31.2 y_3^2 = 388 \left( \frac{200}{y_3} - 6.67 \right)$$

$$\therefore y_3 = 1 \text{ ft}$$

$$E_2 = E_3 + h + h_L$$

$$y_2 + \frac{V_2^2}{2g y_2} = y_3 + \frac{V_3^2}{2g y_3} + 1 + h_L \Rightarrow h_L = 1 \text{ ft}$$

