

# Revision (Mid-Term)



## \* Virtual Work:

$$\Delta = \frac{1}{EI} \int M_0 M_1 dx$$

فحين

$M_0$  : المرسمة او D.M.B للمنت تدرية الاعمال  
الحقيقية (Super Position) او

$M_1$  : تدرية الاعمال و وضع كل مقدار  $H$  و  $H$  عند  
مقدار  $H.m$  من المثل

### أنواع المطالب

Displacement (انزاحة)

و خاضعة الحالة تفر  
الاعمال و وضع مقدار  
 $H$  رأسى فوطى  
الانزاحة الأساسية و اعني  
لو لمبى الانزاحة الفعلية

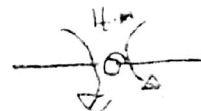
Rotation (دوران)

لو لمبى او Rotation عند نقطة  
تفر الاعمال و وضع عند مقدار  $H.m$

لو لمبى او relative عند

نقطة تتكون من

هذه الحالة وضع عزم  
عكس بعد مقدار  $H.m$  الى اليمين

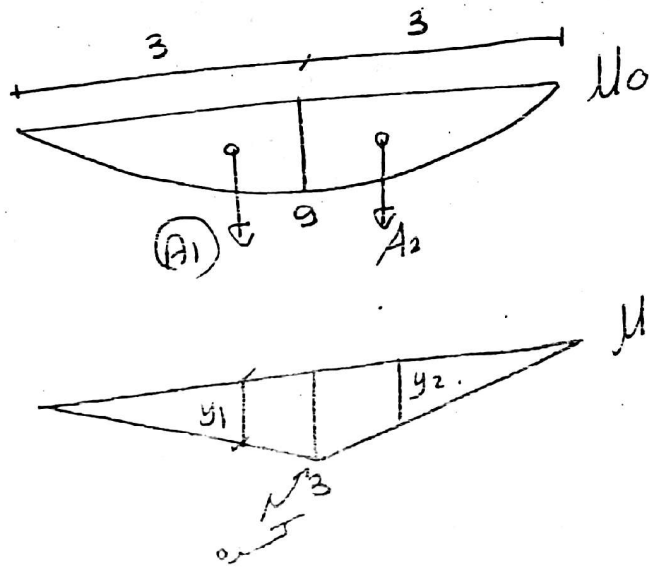


Δ : دل س د س شل س دل

$$\Delta = \frac{1}{EI} \left[ \begin{array}{l} \text{یوخذ کل در کین صا صا صا صا} \\ \text{* لحوول لعا یل صا صا} \\ \text{بار سارا سارا} \end{array} \right]$$

\* لحوقه فایه :  
 حالات و بود کسره فایه صا صا  
 لکات افسره فایه و نکزه یینیا و سارا

Ex:



Consistent deformation...

\* الحلو

① رسم در  $D+D$  & B.M.D. لغات الفريدة

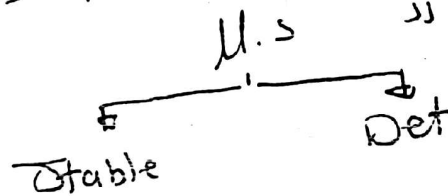
⑤ حساب در deflection و rotation  
نقطه

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\* خطوات حل الحلو

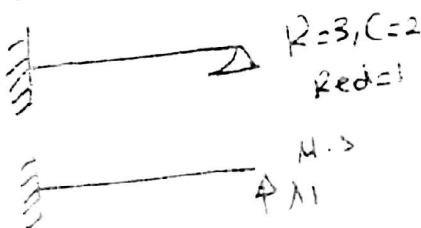
① حساب الجاصل (R) و لغات (C) حبان الجاصل  
② reactions مع حلاقة ان زحل در (الفرد) و لغات  
(SM, SY, SX) و لغات -

③ حبان الجاصل Main system حبان الجاصل لزانة (Redundent)  
مع حلاقة ان يكون در M.S

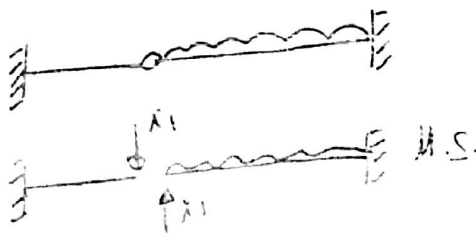


- اختيار در M.S

نقل الجاصل حبان الجاصل حبان الجاصل  
Reaction خارج



نقل الجاصل حبان الجاصل حبان الجاصل  
نظرة حبان الجاصل



۲) نفع  $X_1=0.5$  و درسم (B.M.D) نسبت به آزاد خالص  
 رابطه در Superposition (M.L)

۳) نفع از آزاد و نفع  $X_1=H$  و  $X_1=H \cdot m$  و درسم  
 (M.L)

۵) استویض ایجاد در  $(X_1)$

$$\delta_{10} + X_1 \delta_{11} = 0.5$$

$$\delta_{10} = 1/EI \int M_0 M_1 dx$$

$$\delta_{11} = 1/EI \int M_1 M_1 dx$$

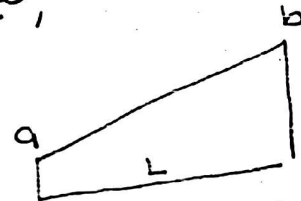
در ایجاد در  $\delta_{11}$  بدتره تکامل المانع نفع حدی  
 علامت \* بعد مقابل یک تطبیق (مقارن) اخیر



$$\delta_{11} = l(a)^2$$



$$\delta_{11} = l/3(a)^2$$



$$\delta_{11} = l/3(a^2 + b^2 + ab)$$

۷) نفع  $X_1$  در یک سازه از شکل و درسم  
 SFD & BMD نسبت به آزاد

- در شکل آن به طلب از B.M.D و در  
 در شکل آن به طلب از SFD و در  
 Reaction در ایجاد در Reaction

$$M_f = M_0 + X_1 M_1$$

\* هدف الماتك ان تكون بانك غير محدد من  
الدرجات الحرة وفي هذه الحالة يتم حذف حركتين  
عند اختيار ال (S.D) فيصبح لدينا  $(X_1, X_2)$

ملاحظة هامة - تكون الخطوات كالآتي:

(1) نضع  $X_1 = X_2 = 0$  ونستخدم B.M.D الجبرية (Superposition ال)

(2) نأخذ ال حالات ونضع  $X_1 = 1$  و  $X_2 = 0$  ونحسب ال

(3) نأخذ ال " ونضع  $X_1 = 0$  و  $X_2 = 1$  ونحسب ال

(4) نستخرج المعادلات الآتية لاختيار  $X_1$  و  $X_2$

$$\begin{aligned} \delta_{10} + X_1 \delta_{11} + X_2 \delta_{12} &= 0 \\ \delta_{20} + X_1 \delta_{21} + X_2 \delta_{22} &= 0 \end{aligned}$$

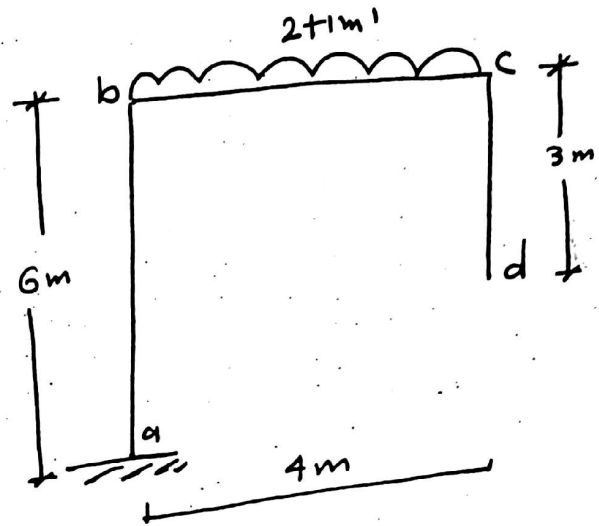
where,

$$\begin{aligned} \delta_{10} &= \frac{1}{EI} \int M_0 M_1 dx \\ \delta_{20} &= \frac{1}{EI} \int M_0 M_2 dx \\ \delta_{11} &= \frac{1}{EI} \int M_1 M_1 dx \quad \text{بالقوانين المحفوظة} \\ \delta_{22} &= \frac{1}{EI} \int M_2 M_2 dx \\ \delta_{12} &= \delta_{21} = \frac{1}{EI} \int M_1 M_2 dx \end{aligned}$$

نحل ال or:  $M_f = M_0 + X_1 M_1 + X_2 M_2$

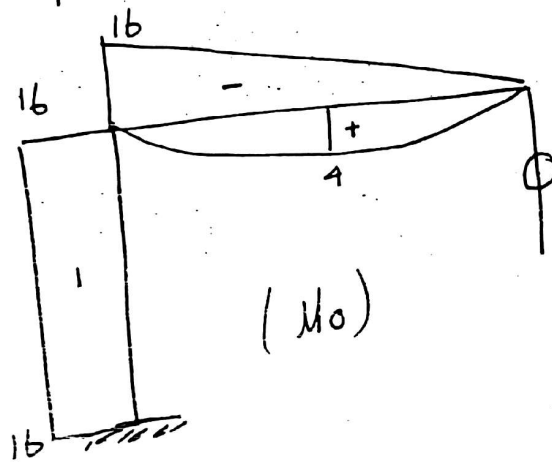
Ex:1 (Virtual work):

Compute The horizontal and vertical deflection at Point (d) of The frame shown if  $EI = 20000 \text{ t.m}^2$ .

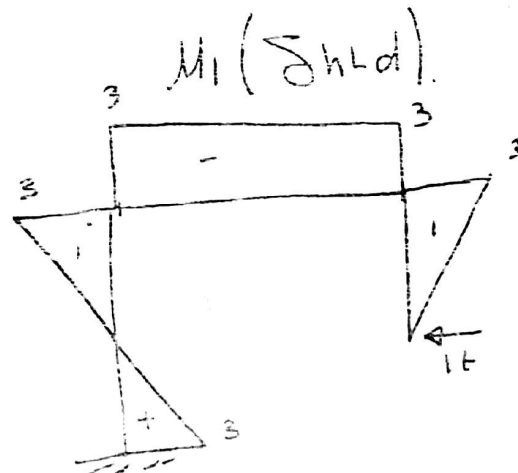
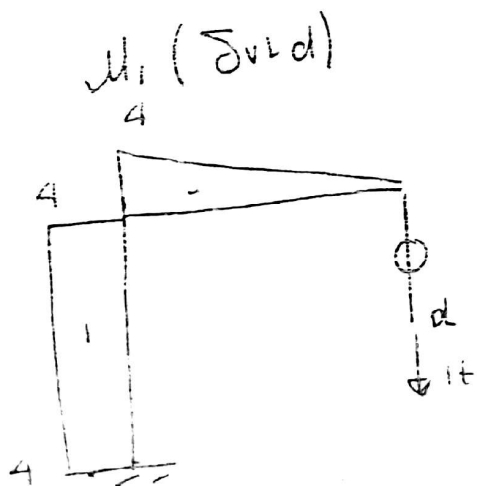


→ Sol →

1) رسم رسم ال B. لتبسيط الزوايا (ملاحظة) (Mo)



2) رسم الزوايا (ملاحظة) (Mo)



$$\Delta_{ld} = 1/EI \left[ \begin{aligned} &(0.5 * 4 * 16 * \frac{2}{3} * 4) - \\ &(2/3 * 4 * 4 * 2) \\ &+ (16 * 6 * 4) \end{aligned} \right]$$

$$= \frac{490.67}{EI}$$

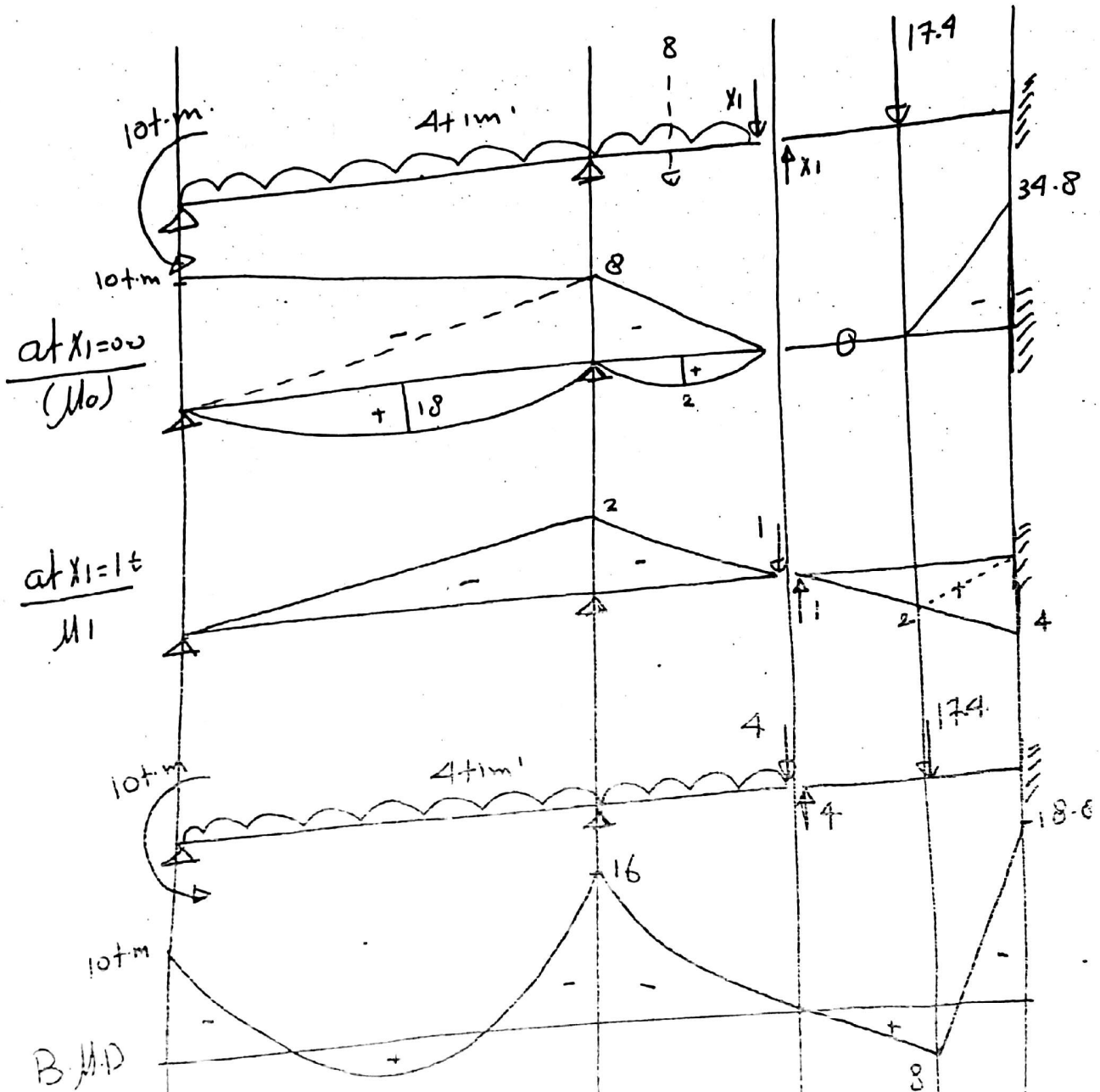
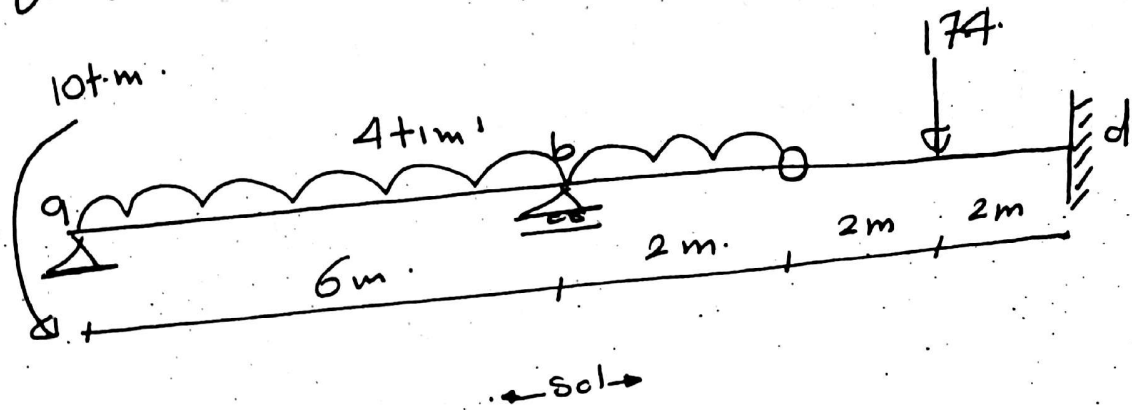
$$\Delta_{hd} = 1/EI \left[ \begin{aligned} &(0.5 * 4 * 16 * 3) \\ &-(2/3 * 4 * 4 * 3) \end{aligned} \right]$$

$$= \frac{64}{EI}$$

EX: 3

using Consistent deformation, Draw The B.M.D diagrams For The Shown beam.

$$\begin{aligned} P &= 4 \\ C &= 2+1=3 \\ \frac{P}{12C} &= 1 \end{aligned}$$





$$\delta_{10} = 1/EI \left[ \begin{aligned} &+ (0.5 \times 6 \times 8 \times (4/3)) + (0.5 \times 6 \times 10 \times (2/3)) \\ &- (2/3 \times 6 \times 18 \times 1) + (0.5 \times 2 \times 8 \times (4/3)) \\ &- (2/3 \times 2 \times 2 \times 1) - (1/2 \times 2 \times 34.8 \times (9.33)) \end{aligned} \right]$$

$$= \frac{-128}{EI}$$

$$\delta_{11} = 1/EI \left[ \begin{aligned} &6/3(2)^2 + 2/3(2)^2 \\ &+ 4/3(16) \end{aligned} \right] = \frac{32}{EI}$$

$$\delta_{10} + X_1 \delta_{11} = 0.0$$

$$\frac{-128}{EI} + X_1 \left( \frac{32}{EI} \right) = 0.0$$

$$\therefore X_1 = 4t$$

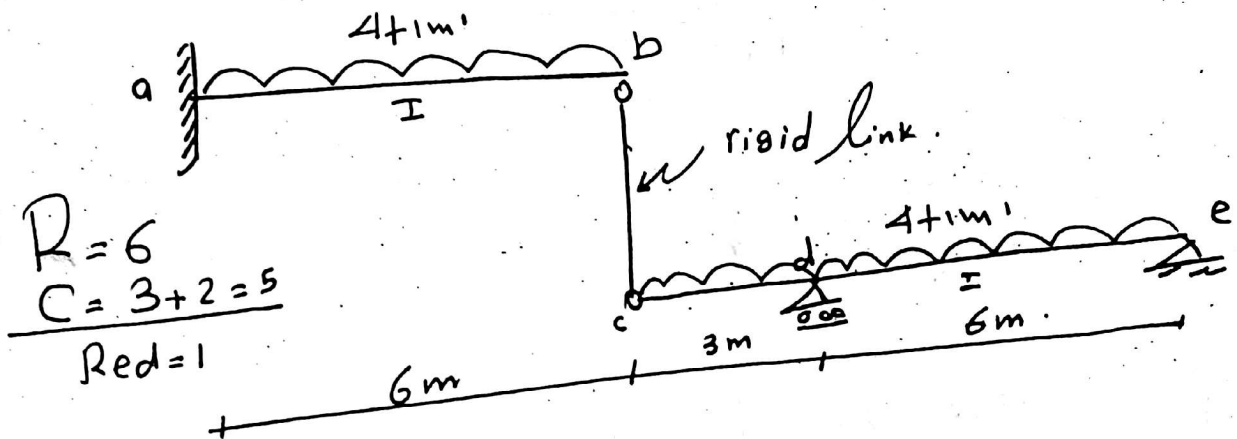
$$\therefore M_f = M_0 + X_1 M_1$$

$$\therefore M_f = M_0 + 4M_1$$

و نفع  $X_1$  مني را عدد  $4t$  در  $M_1$  ميان مقرر  
تغیر امدت یا تغییر القابله دو حدی که در تطبیق است

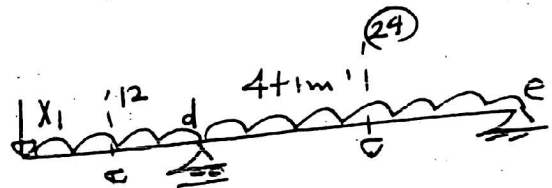
Ex:

Draw B.M.D & S.F.D for The Shown beam using Consistent deformation.

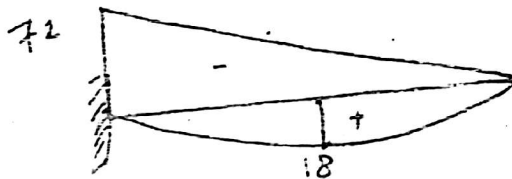


← Sol →

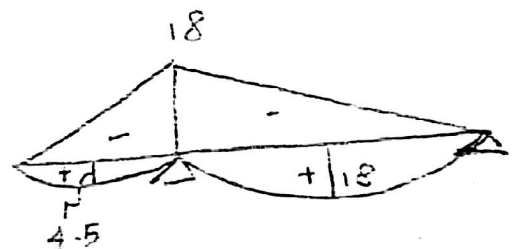
M.S:



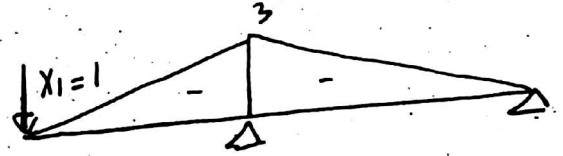
at  $X_1 = 0.0$ ,  $M_0$



$M_0$



at  $X_1 = H$ ,  $\mu_1$ :



$$\delta_{10} = 1/EI \left[ \begin{aligned} &-(0.5 \times 6 \times 72 \times 4) + (2/3 \times 6 \times 18 \times 3) \\ &+ (0.5 \times 3 \times 18 \times 2) - (2/3 \times 3 \times 4.5 \times 1.5) \\ &+ (0.5 \times 6 \times 18 \times 2) - (2/3 \times 6 \times 18 \times 1.5) \end{aligned} \right]$$

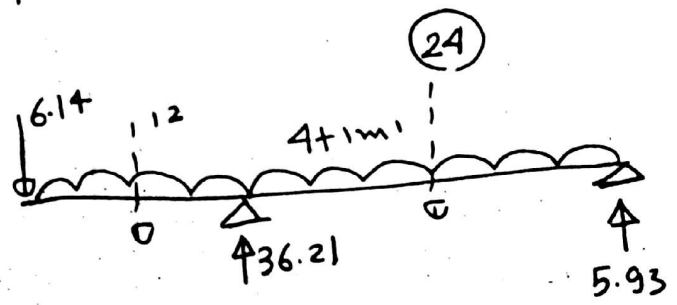
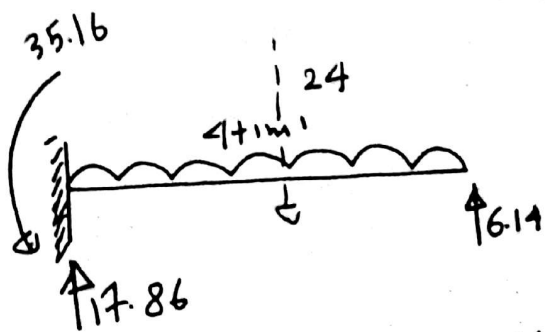
$$= \frac{-607.5}{EI}$$

$$\delta_{11} = 1/EI \left[ \begin{aligned} &6/3(36) + 3/3(9) \\ &+ 6/3(9) \end{aligned} \right] = \frac{99}{EI}$$

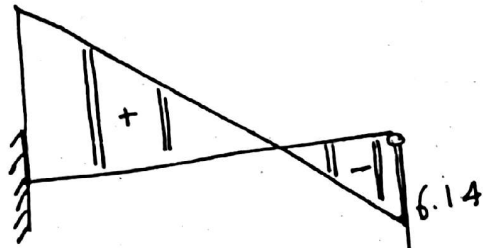
$$\delta_{10} + X_1 \delta_{11} = 0.0$$

$$\frac{-607.5}{EI} + X_1 \frac{99}{EI} = 0.0$$

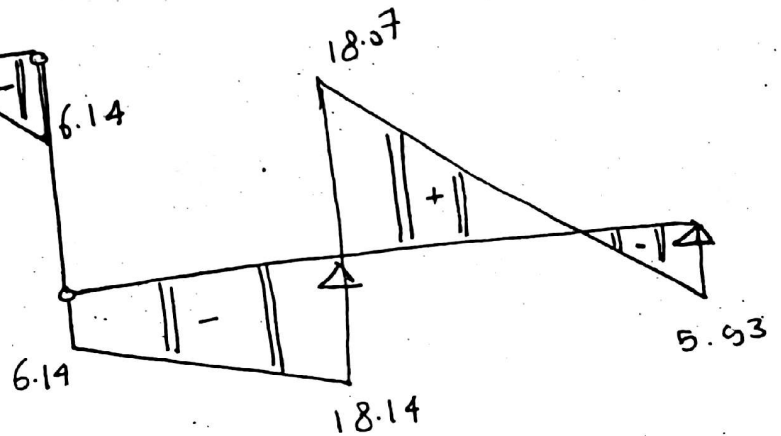
$$\therefore X_1 = 6.14 \text{ t}$$



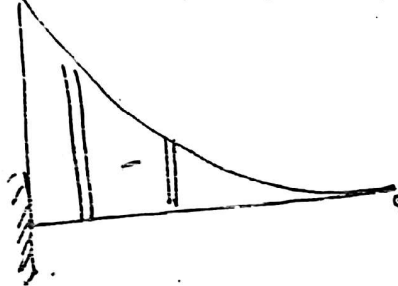
17.86



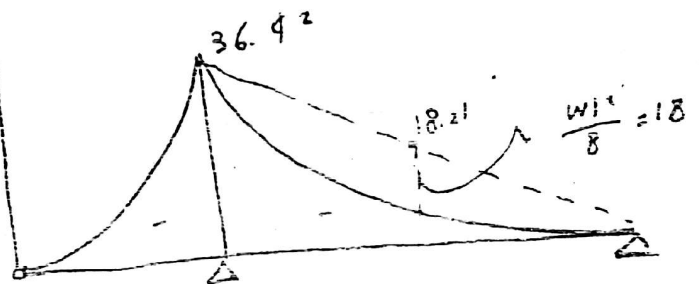
S.F.D



35.16

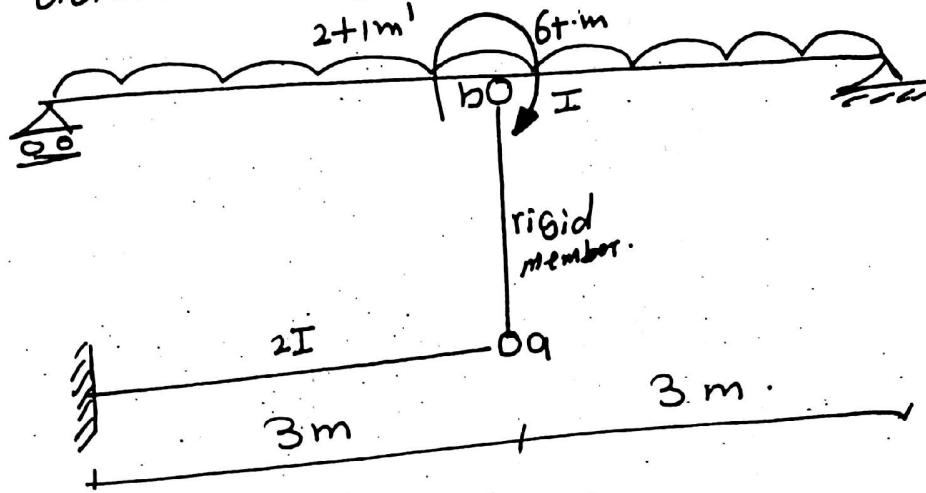


B.M.D



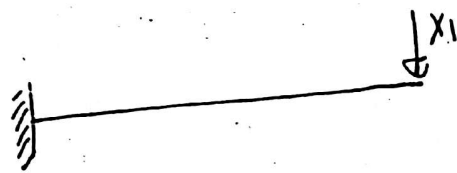
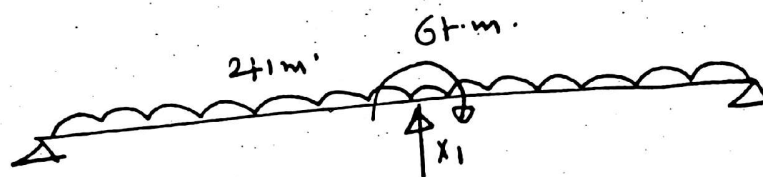
Ex:

For the given structure, draw the S.F.D  
- determine The deflection at Point (a).

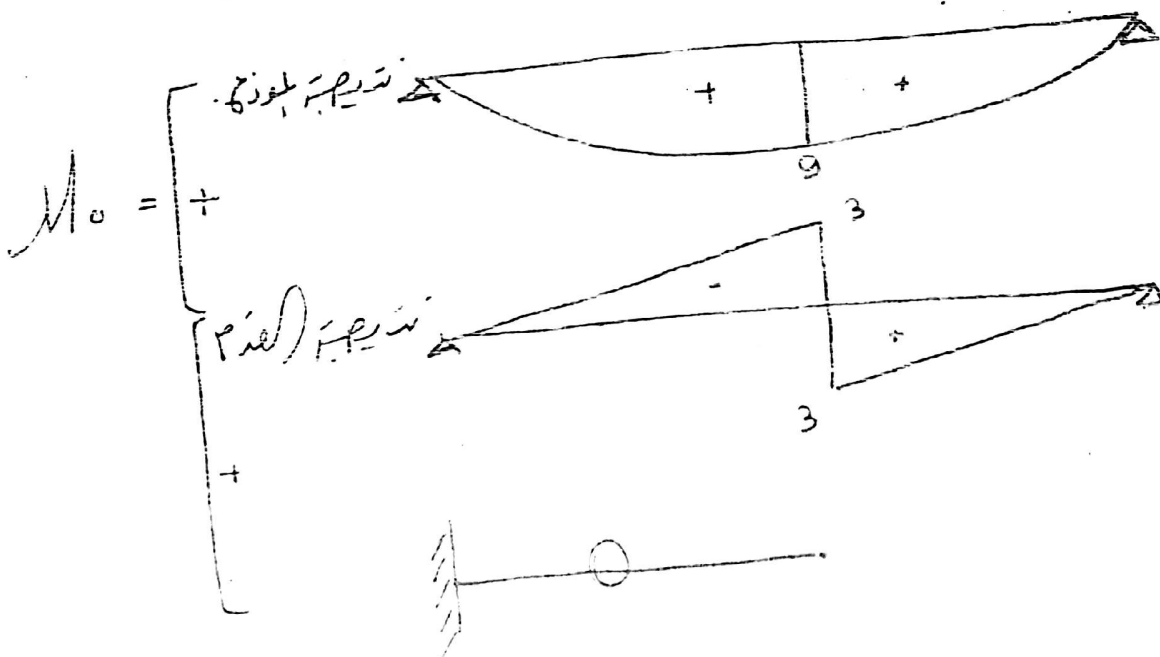
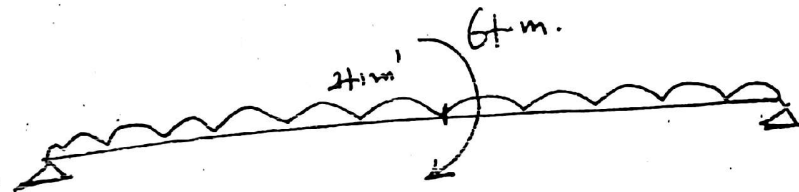


← Sol →

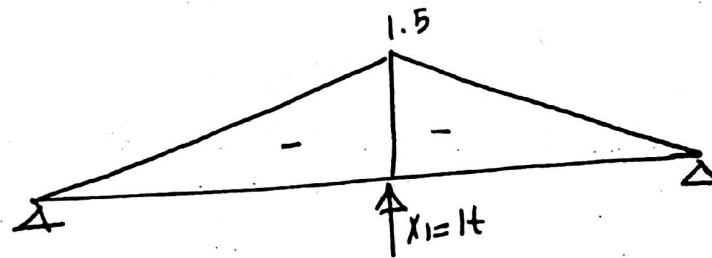
U.S.:



at  $X_1 = 0.0$ ,  $M_0$



at  $X_1 = 1t$ ,  $M_1$ :



$(M_1)$



$$\delta_{10} = \frac{1}{EI} \left[ \left( \frac{2}{3} \times 3 \times 9 \times 2 \right) \times \left( \frac{5}{8} \times 1.5 \right) \right]$$

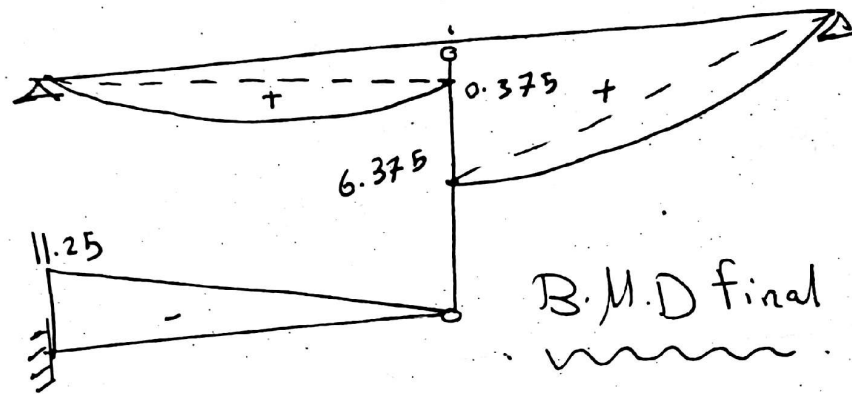
$$= \frac{-33.75}{EI}$$

$$\delta_{11} = \frac{1}{EI} \left[ \frac{3}{3} (1.5)^2 \times 2 + \frac{3}{3} (9) \right] = \frac{9}{EI}$$

$$\delta_{10} + X_1 \delta_{11} = 0$$

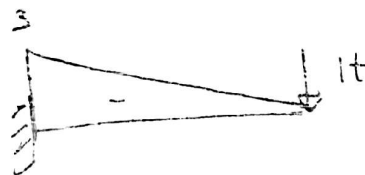
$$\frac{-33.75}{EI} + X_1 \frac{9}{EI} = 0$$

$$\therefore X_1 = +3.75t$$



تلف الايمان وضع  $\alpha = 0.05$  وضع اجل وقدر رأس  
لا مثل عندئذ  $a$  ونسبة  $M_i^*$

$$\Delta a = 1/EI \int M_f M_i^* dx$$

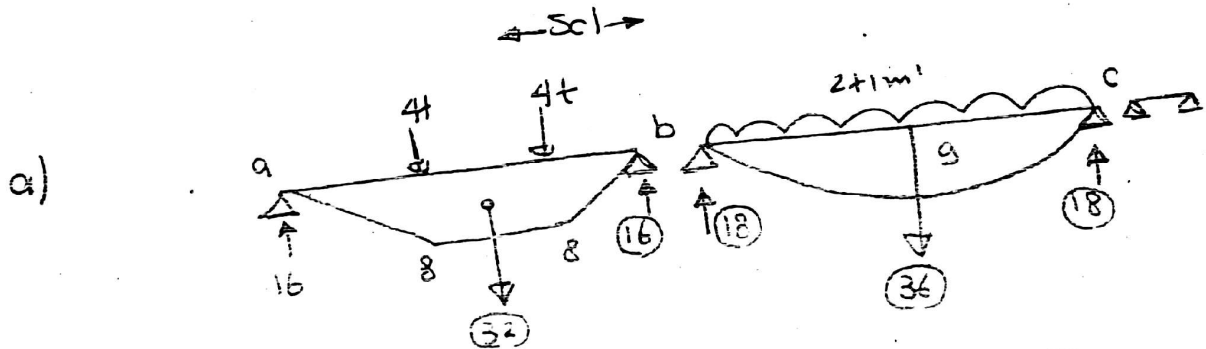
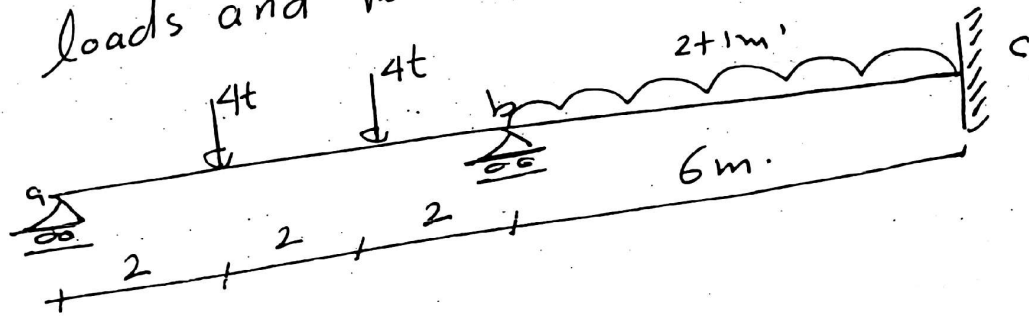


$$\sigma_a = \frac{1}{2EI} [0.5 \times 3 \times 11.25 \times 2] = \frac{16.875}{EI}$$

Ex:

using 3. M equ.

- draw B.M.D due to the applied loads.
- in the absence of loads draw B.M.D due to vertical settlement at  $b = 1 \text{ cm}$  ( $EI = 6000 \text{ m}^2 \cdot \text{t}$ ).
- Find the required vertical settlement at support (b) such that the negative moment at support be equal to the maximum positive moment through the span  $ab$  due to the applied loads and vertical settlement at support (b).



3. M equ at b:

$$0.0 + 2M_b(6+6) + M_c(6) = -6[16+18]$$

$$24M_b + 6M_c = -204 \quad \text{--- (1)}$$

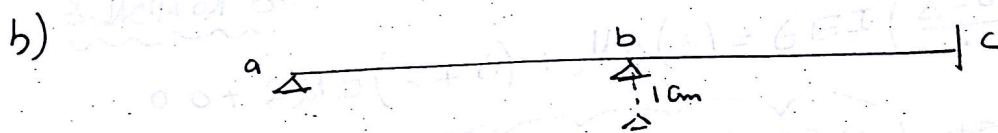
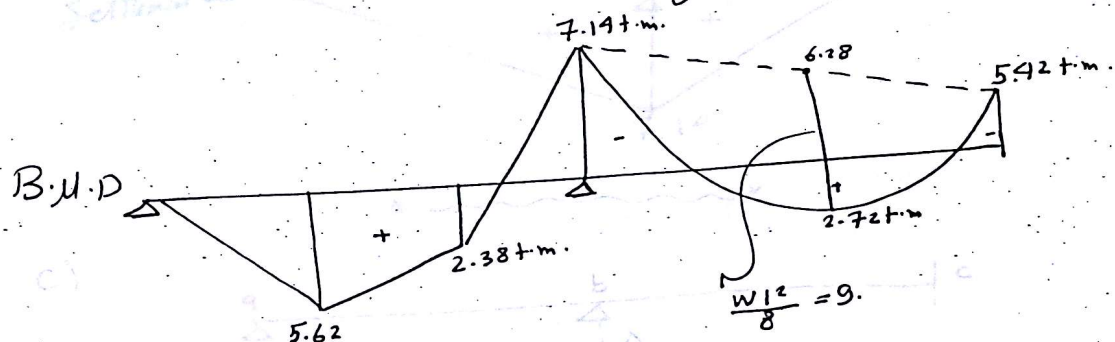
3. M equ at c:

$$M_b(6) + 2M_c(6+0) + 0.0 = -6[18+0]$$

$$6M_b + 12M_c = -108 \quad \text{--- (2)}$$



by solving ① & ② get  $M_b = -7.14 \text{ t.m.}$   
 $M_c = -5.42 \text{ t.m.}$



3. Mequ at b:

$$0.0 + 2M_b \left( \frac{6}{I} + \frac{6}{I} \right) + M_c \left( \frac{6}{I} \right) = 6E \left( \frac{0.01 - 0}{6} + \frac{0.01 - 0.0}{6} \right)$$

$$24M_b + 6M_c = 6EI \left( \frac{0.02}{6} \right)$$

$$= 6 \times 60000 \times \frac{0.02}{6} = 120$$

$$\therefore 24M_b + 6M_c = 120 \rightarrow \text{①}$$

3. Mequ at c:

$$M_b (6) + 2M_c (6 + 0) + 0 = 6EI \left( \frac{0 - 0.01}{6} \right)$$

$$6M_b + 12M_c = -60 \rightarrow \text{②}$$

$$M_b = +7.14, M_c = -8.57$$

(17)

3. Moment:

$$M_a(6) + 2M_a(6+6) + 0.0 = -6(4P_1 + 32)$$

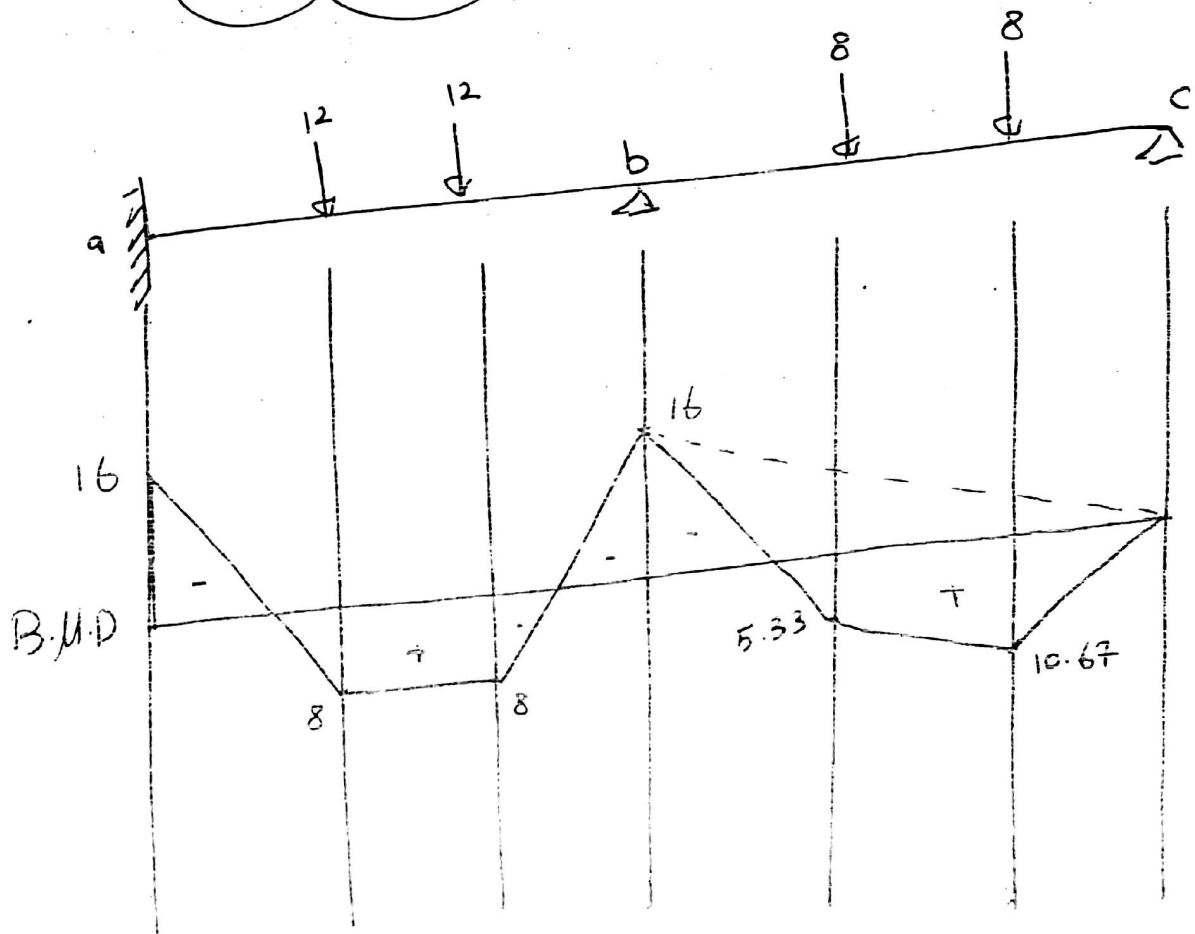
$$6M_a + 24M_a = -24P_1 - 192$$

$$-40P_1 + 24P_1 = -192$$

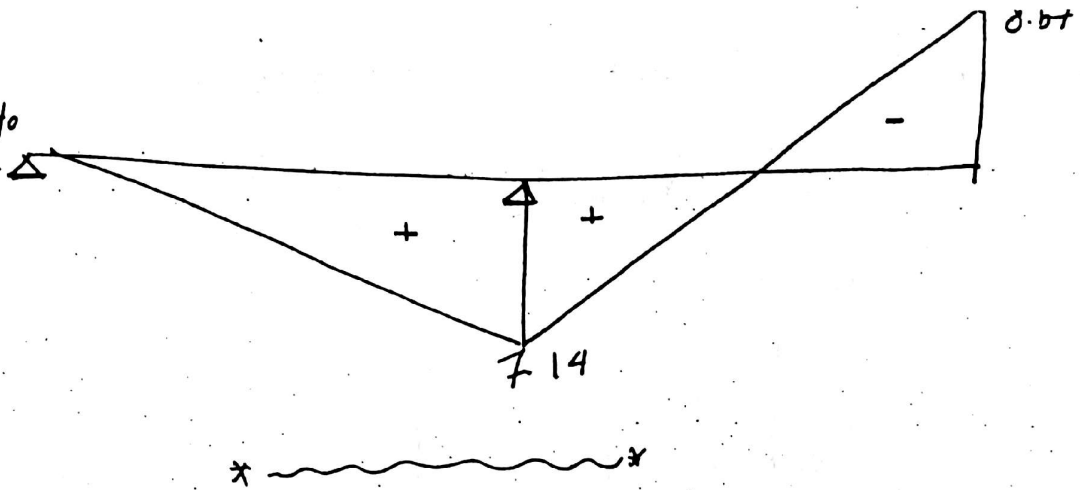
$$\therefore -16P_1 = -192$$

$$\therefore P_1 = 12 \text{ t}$$

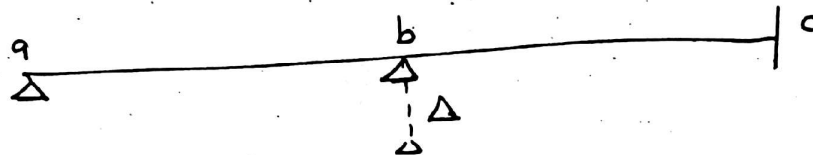
$$\therefore M_a = M_b = -16 \text{ t.m}$$



B.M.D due to Settlement  $\Delta$



c)



3. Eqn at b:

$$0.0 + 2M_b(6+6) + M_c(6) = 6EI \left( \frac{\Delta-0}{6} + \frac{\Delta-0}{6} \right)$$

$$24M_b + 6M_c = 12000\Delta \rightarrow (1)$$

3. Eqn at c:

$$M_b(6) + 2M_c(6+0) + 0.0 = 6EI \left( \frac{0-\Delta}{6} + 0 \right)$$

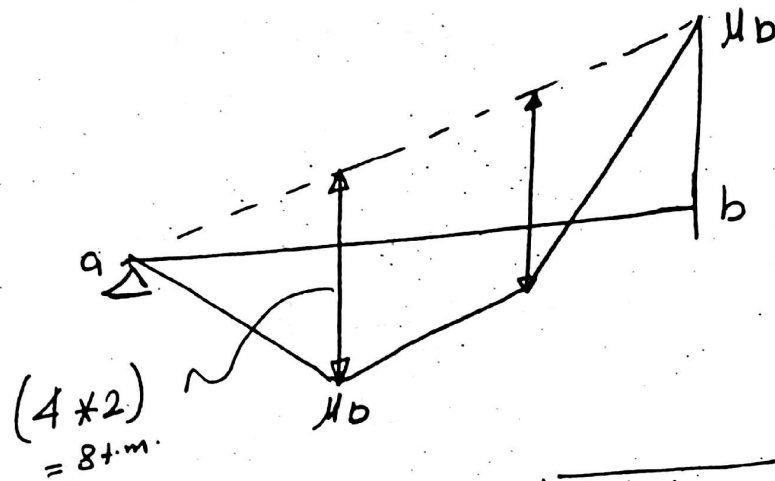
$$6M_b + 12M_c = -6000\Delta \rightarrow (2)$$

by solving (1) & (2) get

$$M_b = 714.28\Delta$$

$$M_c = -857.14\Delta$$

$\therefore$  Negative moment at B =  $\text{Max}_{B, \text{min}}^{\text{+ve}}$  Span ab  
 due to (loads and settlement at b).



$$\therefore M_b = 8 - \frac{1}{3} M_b$$

$$\therefore M_b = 6 \text{ kN}$$

$\rightarrow \therefore \text{B.M.D due to loads only} = -7.14$

$\therefore \text{B.M.D due to settlement at b} = +1.14$

بجانب  
 السطح b عن  
 السطح a  
 السطح b عن  
 السطح a

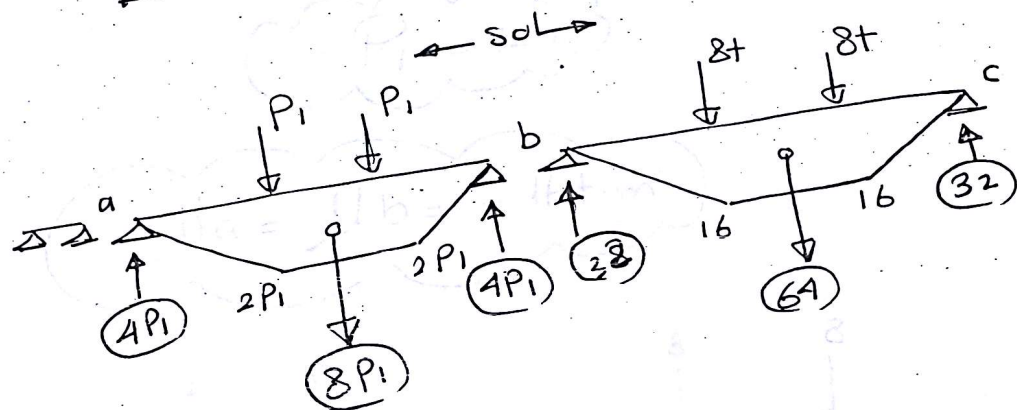
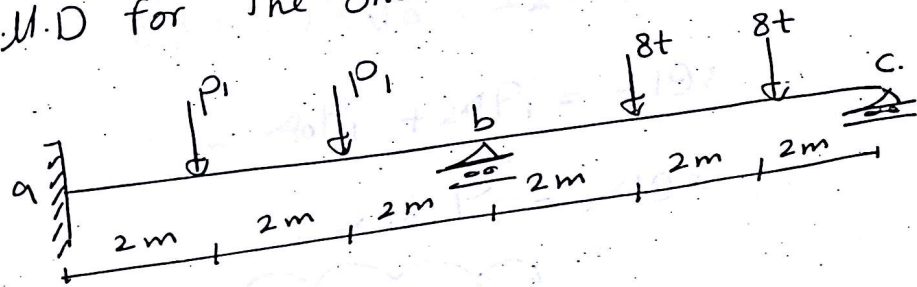
$\therefore M_b \text{ due to settlement} = 714.28 \Delta$

$\therefore 714.28 \Delta = 1.14$

$\therefore \Delta = 1.596 \times 10^{-3} \text{ m}$



Ex: 2 using 3.Moment eq. Find The Value of  $P_1$  Such That The B.M at (a) equals The B.M at b in Magnitude and sign ( $M_a = M_b$ ), further draw B.M.D for The shown beam.



3.Mat a:

$$0.0 + 2M_a(0.0 + 6) + M_b(6) = -6(0 + 4P_1)$$

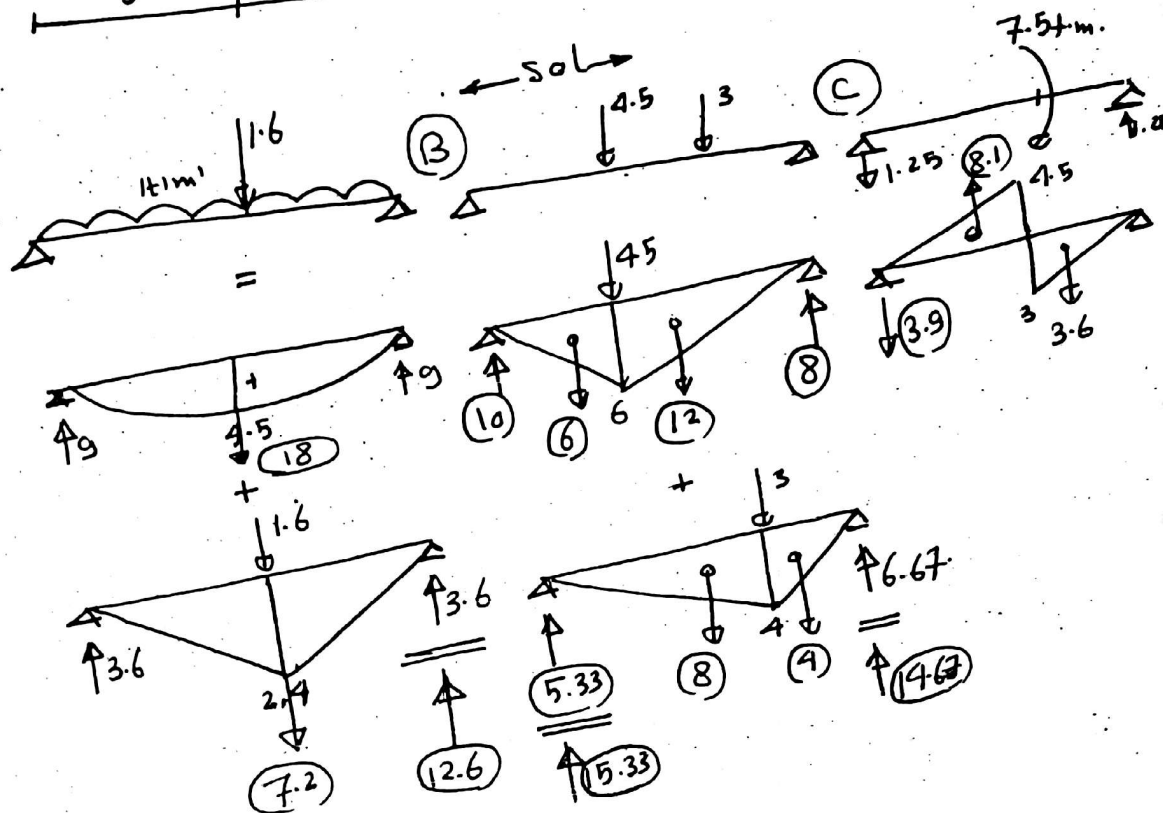
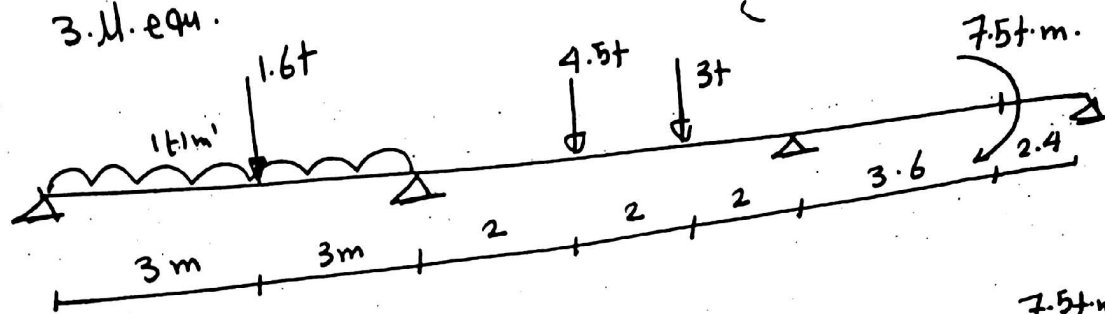
$$12M_a + 6M_b = -24P_1$$

but  $M_a = M_b$

$$\therefore 18M_a = -24P_1$$

$$\therefore M_a = -1.33P_1$$

Draw S.F.D & B.M.D for the shown beam. 3.M. eqn.



3. Meq at B:

$$0.0 + 2M_B(6+6) + M_C(6) = -6(12.6 + 15.33)$$

$$24M_B + 6M_C = -167.58 \quad \text{--- (1)}$$

3. Meq at C:

$$M_B(6) + 2M_C(6+6) + 0.0 = -6[14.67 + (-3.9)]$$

$$6M_B + 24M_C = -64.62 \quad \text{--- (2)}$$

by solving (1) & (2) get

$$\begin{cases} M_B = -6.73 \text{ t.m} \\ M_C = -1.01 \text{ t.m} \end{cases}$$

